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56 Ö REFERENCE

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# STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

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# **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY \_GRANVILLE

PROJECT DESCRIPTION BRIDGE NO. 42 ON SR 1724 (NORTHSIDE RD.) OVER LEDGE CREEK

STATE PROJECT REFERENCE NO. B-5677

### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-680. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS OF THE INVESTIGATION. THE STATEM LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE INVESTIGATION. THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS AND ASSECTIONS OF THE ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AS WELL AS A CALLED NOW CHARTSE CACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- IES:
  THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT
  OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS
  OR CONTRACT FOR THE PROJECT.
  BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS
  FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
  CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

TRIGON GOODNIGHT, D. J. DRAWN BY \_\_HUNSBERGER, W. S. CHECKED BY HAMM, J. R. SUBMITTED BY FALCON ENG. DATE \_OCTOBER 2017

PERSONNEL



**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

PROJECT REFERENCE NO. SHEET NO. 2

# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION	GRADATION	ROCK DESCRIPTION	TERMS AND DEFINITIONS		
SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT	<u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.  UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.	HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.	ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.		
ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING:	GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN	AQUIFER - A WATER BEARING FORMATION OR STRATA.		
CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	REPRESENTED BY A ZONE OF WEATHERED ROCK, ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.		
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	WEATHERED WEATHERED NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES >	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.		
SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT		
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS ORGANIC MATERIALS	MINERALOGICAL COMPOSITION  MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC.	CRYSTALLINE CRYSTALLINE WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE.	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.		
CLASS. (≤ 35% PASSING *200) (> 35% PASSING *200) (> 36% PASSING *200) (> 36% PASSING *200) (> 36% PASSING *200)	ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	GNEISS, GABBRO, SCHIST, ETC.	CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.		
CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-7-6 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED.	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM		
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50	ROCK (NCR)  ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.  COASTAL PLAIN  COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD	OF SLOPE.		
7. PASSING	HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.		
*10 58 MX 58 MX 51 MN GRANULAR CLAY MUCK, *40 38 MX 58 MX 51 MN PEAT	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT		
■ 2000   15 MX   25 MX   10 MX   35 MX   35 MX   35 MX   36 MN   36 M	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.		
MATERIAL	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	<u>DIP</u> - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.		
PASSING *40 40 MX 41 MN 40 MX 41 MN 40 MX 41 MN 48 MX 41 MN 48 MX 41 MN LITTLE OR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE		
PI 6 MX NP IW MX IW MX II MN II MN IW MX IW MX II MN II MN MODERATE OPCONIC	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE  GROUND WATER	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH,  FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE		
GROUP INDEX 0 0 4 MX 8 MX 12 MX 16 MX NU MX AMUUNTS UF SOILS	SHOOKS WATER	SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO  (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.		
USUAL TYPES STONE FRAGS.  OF MAJOR GRAYEL AND GAND GAND GRAYEL AND GAND GAND GAND GAND GRAYEL AND GAND GAND GAND GAND GAND GAND GAND	WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING	CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.		
MATERIALS SAND SAND CRAVEL AND SAND SOILS SOILS	STATIC WATER LEVEL AFTER 24 HOURS	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN  (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	<u>FLOAT</u> - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.		
GEN, RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.		
PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	WITH FRESH ROCK.  MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE		
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH	FIELD.		
PRIMARY SOIL TYPE COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	(MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES 'CLUNK' SOUND WHEN STRUCK,  IF TESTED, WOULD YIELD SPT REFUSAL	JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.		
CONSISTENCY (N-VALUE) (TONS/FT <sup>2</sup> )	WITH SOIL DESCRIPTION OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT	<u>LEDGE</u> - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.		
GENERALLY VERY LOOSE < 4 1005E 4 TO 10	SOIL SYMBOL  SOIL SYMBOL  SPT ONT TEST BORING  SLOPE INDICATOR INSTALLATION	(SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED  TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.		
MATERIAL MEDIUM DENSE 10 TO 30 N/A	M - 151	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF	MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS, MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.		
(NON-COHESIVE) DENSE 30 TO 50  VERY DENSE > 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT TEST	VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE		
VERY SOFT < 2 < 0.25	INFERRED SOIL BOUNDARY CORE BORING    SOUNDING ROD	(V SEV.) REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR	OF AN INTERVENING IMPERVIOUS STRATUM.		
GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	INFERRED ROCK LINE MN MONITORING WELL TEST BORING	VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF  COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.		
MATERIAL STIFF 8 TO 15 1 TO 2 (COHESIVE) VERY STIFF 15 TO 30 2 TO 4	WITH CURE	SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS	ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE		
HARD > 30 > 4	TEZUMETER	ALSO AN EXAMPLE.  ROCK HARDNESS	RUN AND EXPRESSED AS A PERCENTAGE.		
TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.		
U.S. STD. SIEVE SIZE 4 10 40 60 200 270  OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO		
COARSE FINE	SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF	HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.		
BOULDER COBBLE GRAVEL SAND SAND SILT CLAY		MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT		
GRAIN MM 305 75 2.0 0.25 0.05 0.005	ABBREVIATIONS  AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	OR SLIP PLANE.  STANDARD PENETRATION TEST (PENETRATION RESISTANCE)(SPT) - NUMBER OF BLOWS (N OR BPF) OF		
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL		
SOIL MOISTURE - CORRELATION OF TERMS	CL CLAY  MOD MODERATELY  7 - UNIT WEIGHT  CPT - CONE PENETRATION TEST  NP - NON PLASTIC  7 - DRY UNIT WEIGHT	HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.		
SOIL MOISTURE SCALE FIELD MOISTURE (ATTERBERG LIMITS) DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION	CSE COARSE ORG ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK, CAN BE EXCAVATED IN FRAGMENTS	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.		
<u> </u>	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK	FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.			
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY (SAT.) FROM BELOW THE GROUND WATER TABLE	e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL 10 OR TORRAFER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PRICENTAGE.  10PSOIL (T.S.) - SURFACE SOILS USUALLY CONTAINED ORGANE WOLLD		
PLASTIC PLOUID LIMIT	─ FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK	SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.		
RANGE - WET - (W) SEMISOLID; REGULRES DRYING TO	FRAGS FRAGMENTS $w$ - MOISTURE CONTENT CBR - CALIFORNIA BEARING	FRACTURE SPACING BEDDING	BENCH MARK: BM-2: 36' NAIL IN PINE TREE		
(PI) PL PLASTIC LIMIT ATTAIN OPTIMUM MOISTURE	HI HIGHLY V - VERY RATIO	TERM SPACING TERM THICKNESS  VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	N. 856737 F. 2086268		
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET	STA. 16+84 OFFSET: 30' LT -L- STELEVATION: 269:34 FEET 039779		
SL SHRINKAGE LIMIT	CME-45C CLAY BITS X AUTOMATIC MANUAL	MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET	NUTES:		
- DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE	6 CONTINUOUS FLIGHT AUGER	VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET	FIAD - FILLED IMMEDIATELY AFTER DRILLING		
PLASTICITY	CME-55	INDURATION	CO GIVE WE		
PLASTICITY INDEX (PI) DRY STRENGTH	CME-550 HARD FACED FINGER BITS X-N 02	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	MAR. SINY		
NON PLASTIC 0-5 VERY LOW	TUNGCARBIDE INSERTS	RUBBING WITH FINGER FREES NUMEROUS GRAINS; FRIABLE GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	Can V Her 10/5/1+		
SLIGHTLY PLASTIC 6-15 SLIGHT MODERATELY PLASTIC 16-25 MEDIUM	VANE SHEAR TEST X CASING X ADVANCER HAND TOOLS:  POST HOLE DIGGER	CRAINE CAN BE CERARATED FROM CAMBLE WITH CIFEL BRODE.			
HIGHLY PLASTIC 26 OR MORE HIGH	PORTABLE HOIST X TRICONE 2 15/16 STEEL TEETH HAND AUGER	MODERATELY INDURATED BREAKS EASILY WHEN HIT WITH HAMMER.			
COLOR	TRICONE TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE;  INDURATED DIFFICULT TO BREAK WITH HAMMER.			
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).	X CORE BIT VANE SHEAR TEST	CHARD HAMMER BLOWS REGULTRED TO RREAK SAMPLE.			
MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.		EXTREMELY INDURATED SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14		

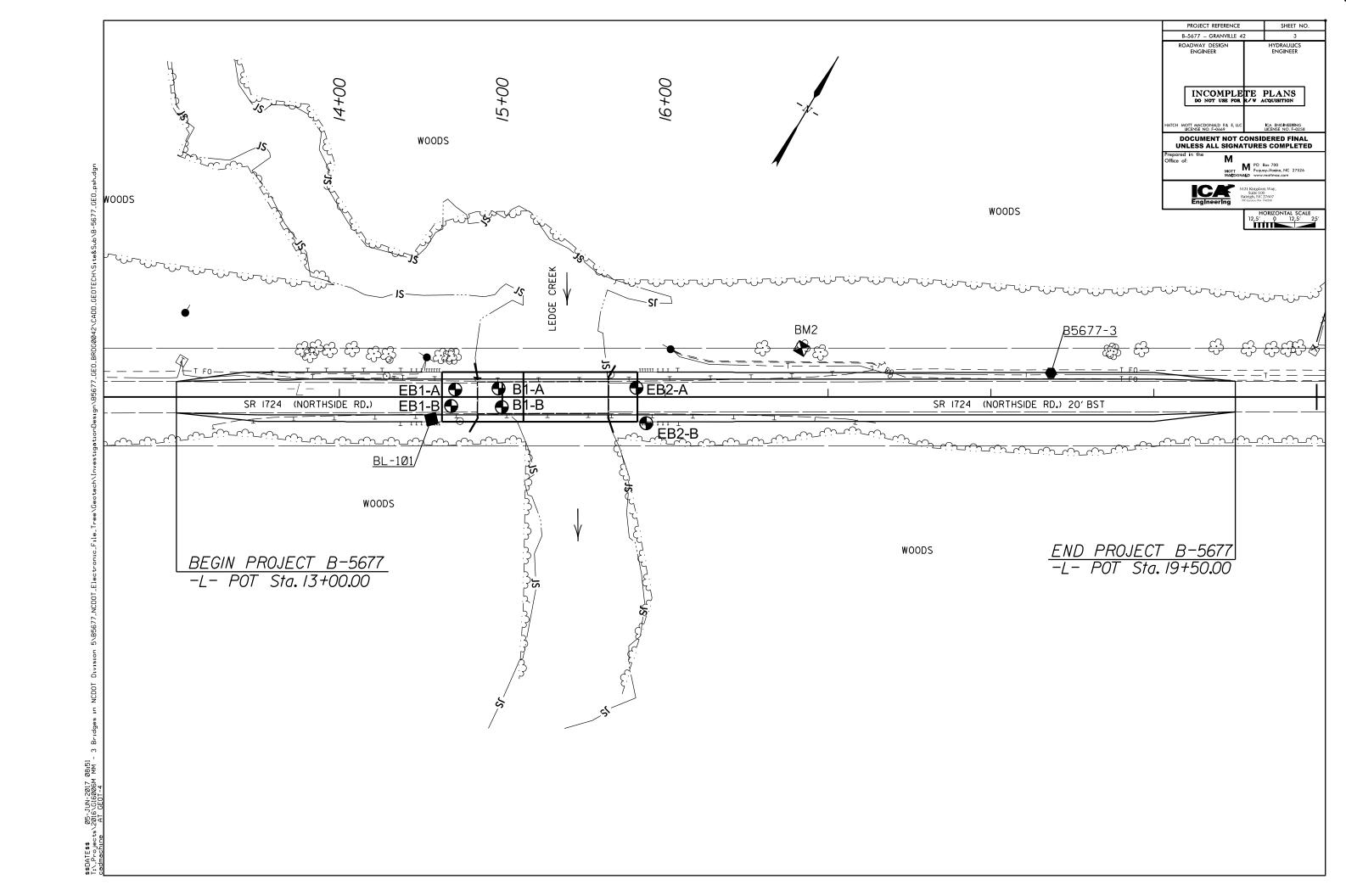
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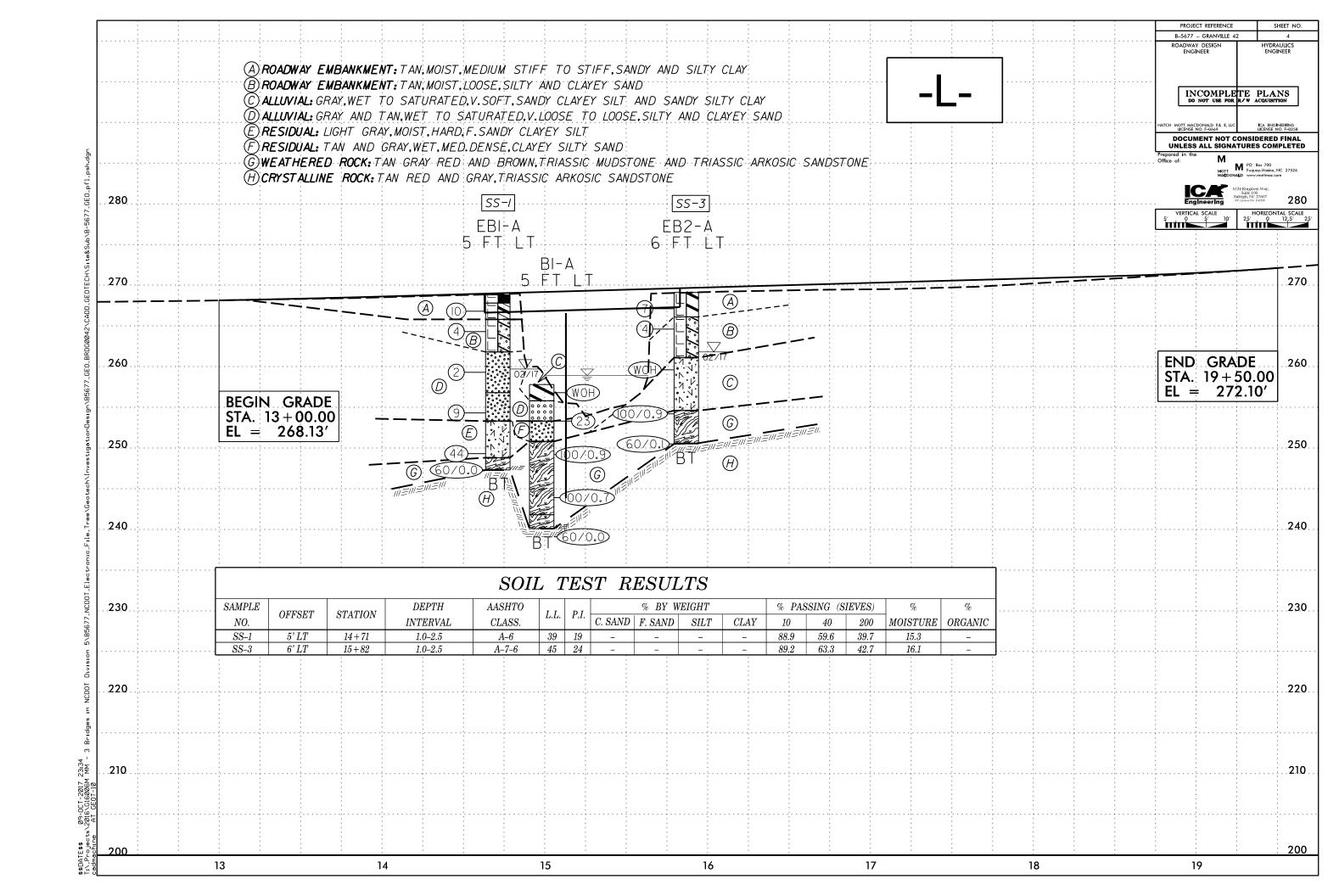
# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

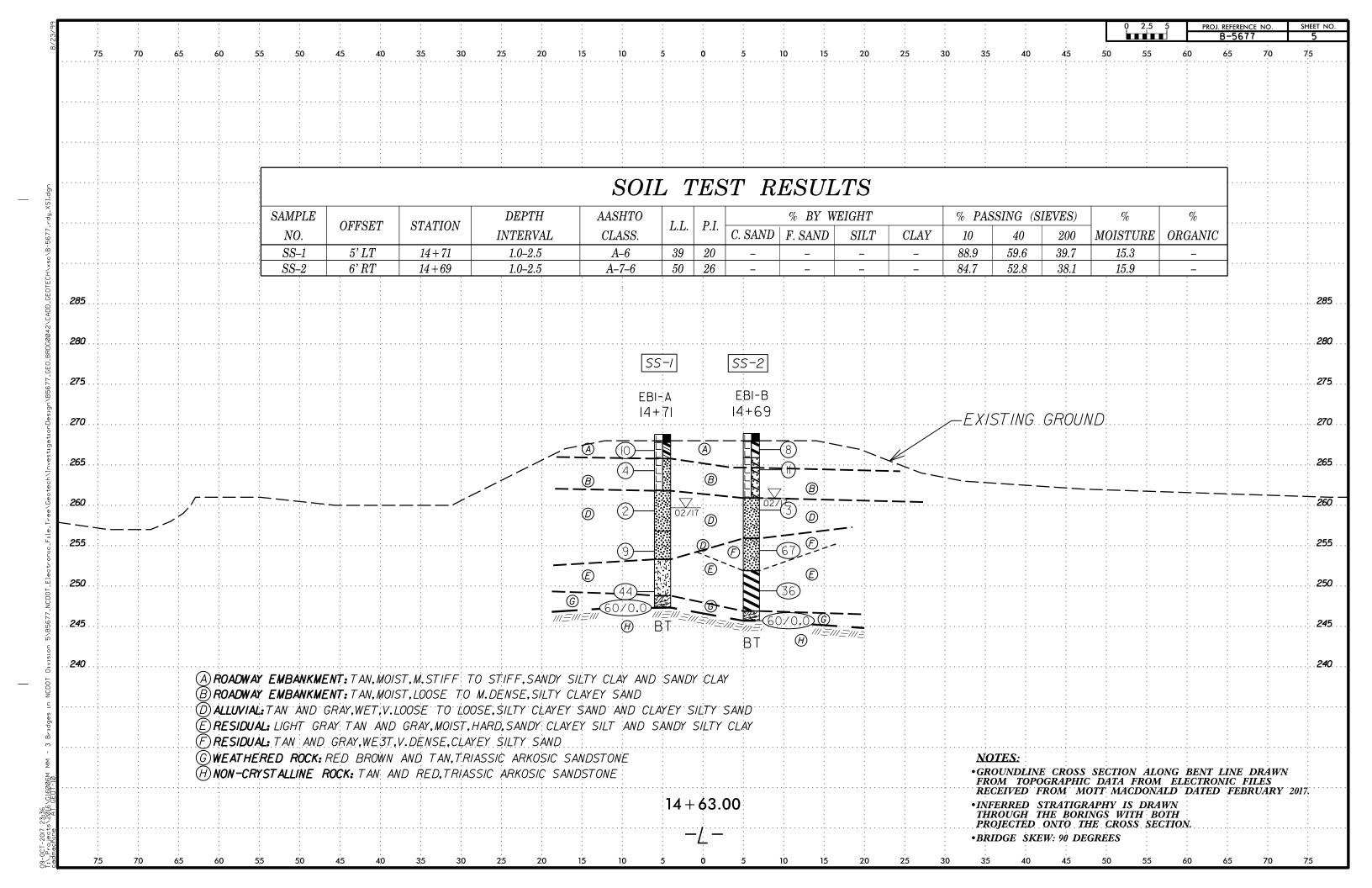
# SUBSURFACE INVESTIGATION

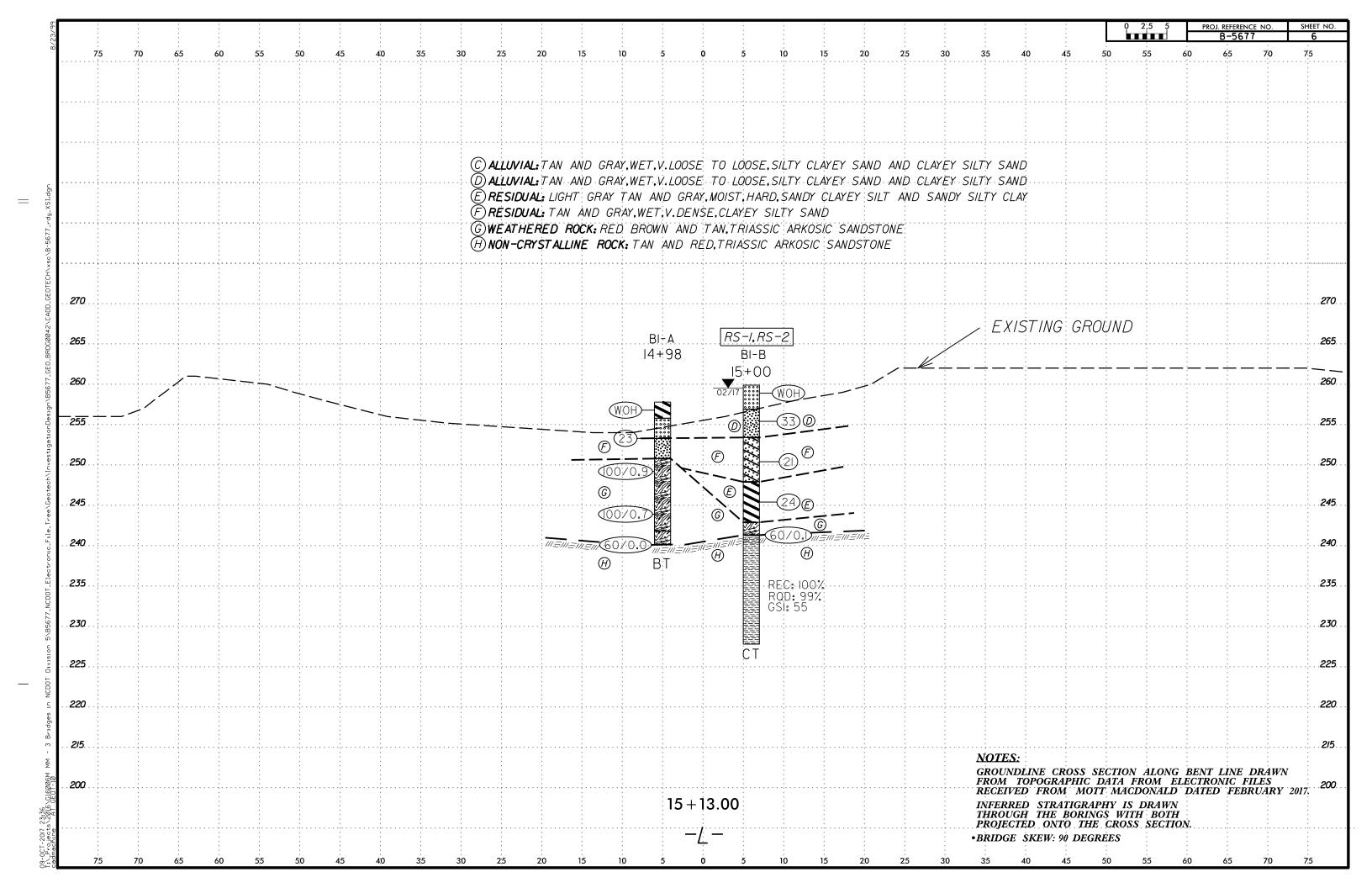
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

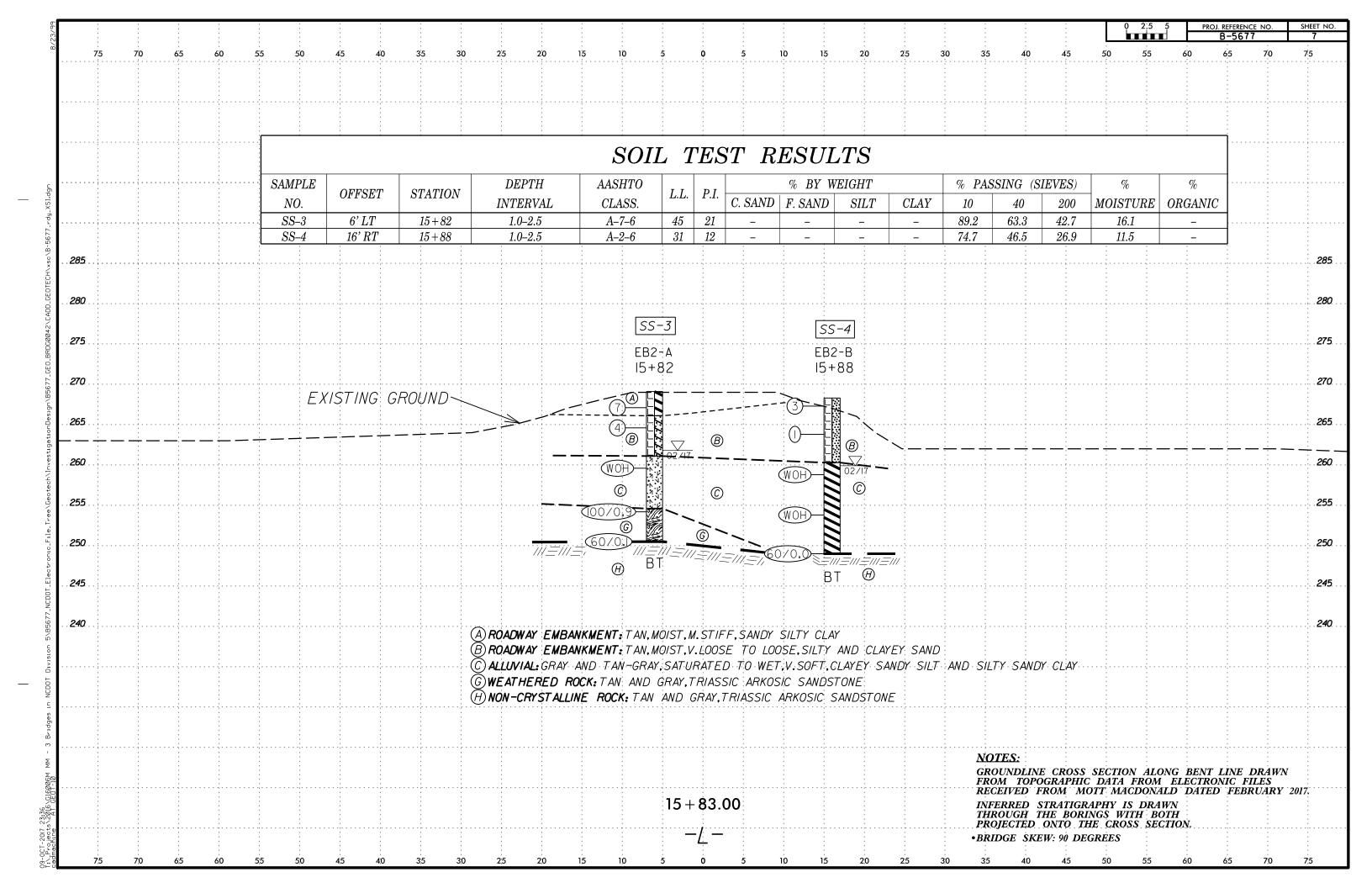
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join	nted Ro	ock Mass (Marinos and Hoek, 2	2000)			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)		ν 0 0		ν Φ Ο	8 9 9 9	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS	VERY GOOD  Very rough, fresh unweathered surfaces  GOOD  Rough, slightly weathered, iron stained surfaces	<b>FAIR</b> Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfa- with compact coatings or fillings or angular fragments	<b>VERY POOR</b> Slickensided, highly weathered surf with soft clay coatings or fillings	Surface conditions (barticinally be appeared surface stress analysis.  WERY COOD - Rough, slightly weathered courtness are beesence of decreased surface surfaces of to lady on the hopfy of to lady on the hopfy of the boat of the strength of surfaces and salight shift to the right in the columns to the allowed by a slight shift to the right in the columns to the lady of the la
STRUCTURE		DECREASING SI	JRFACE QU	ALITY =	>	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities  BLOCKY - well interlocked un-	PIECES 	90 80 70		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass, in shallow tunnels or slopes these bedding planes may cause structurally controlled instability.  A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass, in shallow tunnels or slopes these bedding planes may cause structurally controlled instability.
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets  VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks	OCKING OF ROCK	70 60	50			B. Sand- stone with thin inter- layers of siltstone amounts  D. Siltstone or silty shale with sand- stone layers stone with siltstone or clayey shale with sandstone layers  40
formed by 4 or more joint sets  BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	    ASING INTERLOC 		40	30		C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.  F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	 			20		G. Undisturbed silty or clayey shale with or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone layers
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	٧	N/A N/A		$\langle \ / \ \rangle$	10	Into small rock pieces.   → Means deformation after tectonic disturbance  DATE: 8-19-



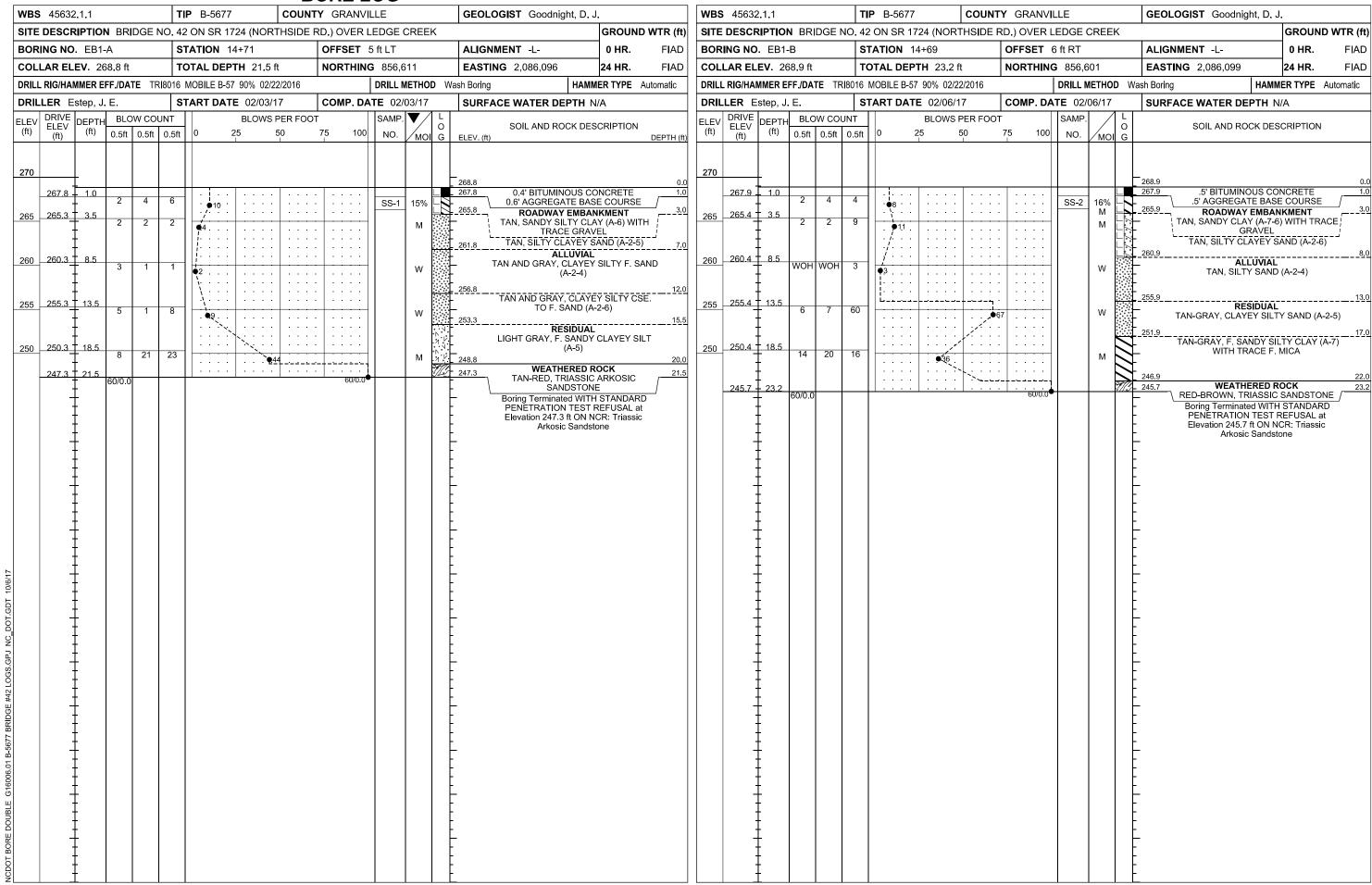








# GEOTECHNICAL BORING REPORT BORE LOG



# GEOTECHNICAL BORING REPORT BORE LOG

											B	JK	EL	<u>.OG</u>			
WBS	4563	2.1.1			TI	<b>P</b> B-56	677			COL	JNTY	<b>'</b> GF	RANVI	LLE			GEOLOGIST Goodnight, D. J.
SITE	DESC	RIPTIO	N BR	IDGE	NO. 42	2 ON SF	₹ 172	24 (N	ORT	HSIE	E R	D.) C	VER I	EDGE	CREE	K	GROUND WTR (
BOR	ING N	<b>)</b> . B1-/	4		S <sup>-</sup>	TATION	<b>l</b> 14	+98				OFF	SET	5 ft LT			ALIGNMENT -L- 0 HR. FIAI
COL	LAR E	<b>LEV</b> . 2	57.8 ft		T	OTAL D	EPT	<b>H</b> 17	7.7 ft			NOF	RTHING	<b>3</b> 856,	625		<b>EASTING</b> 2,086,119 <b>24 HR.</b> FIAI
DRILL	_ RIG/HA	AMMER E	EFF./DA	TE T	RI8016	MOBILE	B-57	90% (	02/22	/2016				DRILL	METHO	D W	Vash Boring HAMMER TYPE Automatic
DRIL		Estep, J	J. E.		S	TART D	ATE	02/0	03/17	7		CON	IP DA	TE 02	/03/17	4	SURFACE WATER DEPTH 1.5ft
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	1	0	2:		WS P	ER F		75 	100	SAMP NO.	MO	L O I G	SOIL AND ROCK DESCRIPTION ELEV. (ft) DEPTH
260		1															_
055	257.8	0.0	WOH	WOH	WOH	•Q_									Sat.		257.8 ALLUVIAL 255.8 GRAY, F. SANDY SILTY CLAY (A-7)2
255	254.3	3.5	12	10	13		```								w	0000	GRAY, SLIGHTLY SILTY SAND (A-3)  253.3  RESIDUAL
250	250.1	7.7	26	74/0.4	1		<u>[</u>	 	· ·	 	 	<u>  :</u>	· · · · · · · · · · · · · · · · · · ·			T)	250.8 GRAY AND TAN, CLAYEY SILTY SAND (A-2-5) WEATHERED ROCK
		Ī											100/0.9				BROWN, TRIASSIC MUDSTONE
245	245.1	12.7	25	60	40/0.2							1 .	100/0.7	•			
	240.1	† 17.7	60/0.0										60/0.0	-			241.8 11 240.1 TAN, TRIASSIC ARKOSIC SANDSTONE 17 Boring Terminated WITH STANDARD PENETRATION TEST REFUSAL at
		+ + + + + + + + + + + + + + + + + + +															Elevation 240.1 ft ON NCR: Triassic Arkosic Sandstone
		† + + + + + + + + + + + + + + + + + + +															

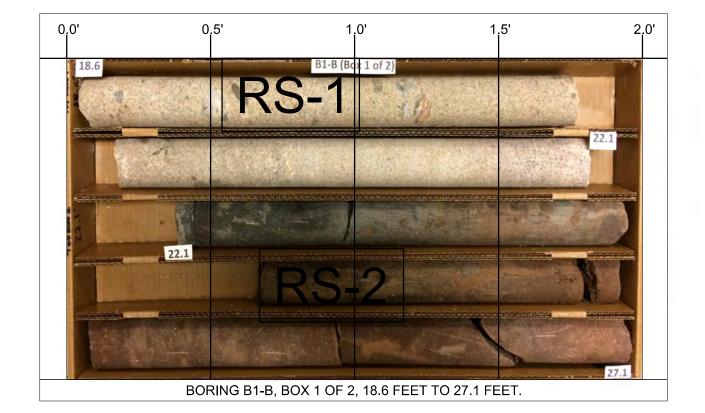
SHEET 9

WBS	45632	.1.1			TI	<b>P</b> B-	5677			COU	NTY	' GF	RANV	ILLI	 E			GEOLOGIST Goodnight, D. J.		
SITE	DESCR	IPTIO	N BRI	DGE	NO. 42	ONS	SR 17	24 (N	IORT	HSID	ERI	D.) C	VER	LE	OGE C	CREE	<	<u> </u>	GROUNE	WTR (ft)
BOR	ING NO.	. B1-B	}		S	TATIC	N 1	5+00				OFF	SET	6 ft	RT			ALIGNMENT -L-	0 HR.	N/A
COLI	LAR ELI	<b>EV.</b> 25	9.9 ft		т	DTAL	DEP	<b>TH</b> 3.	2.1 ft			NOF	RTHIN	G	856,6	20		<b>EASTING</b> 2,086,130	<b>24 HR</b> . (	0.4 Caved
DRILL	. RIG/HAN	IMER E	FF./DA	TE TE	RI8016	MOBIL	E B-57	90%	02/22	/2016				D	RILL M	IETHOI	o w	/ash Boring HAMM	ER TYPE	Automatic
DRIL	LER E	step, J.	E.		S	TART	DATI	E 02/	/02/17	7		CON	/IP. D	\TE	02/0	02/17		SURFACE WATER DEPTH N	A	
ELEV	DRIVE	DEPTH		w co					WS P		TOC			s	SAMP.		L			
(ft)	ELEV (ft)	(ft)	0.5ft	0.5ft	0.5ft	0	2	25	5	0	7	75	100		NO.	мог	O G	SOIL AND ROCK DES	CRIPTION	
260																		259.9		0.0
200	259.9	0.0	1	1	WOH	<b>Q</b> 1										_	0000	ALLUVIAL		
	256.4	- - 3.5				```	``,										0000	TAN, SLIGHTLY SILTY SA 256.9 LITTLE SURFICIAL ASPH		
255	230.4	3.5 - -	1	32	1			` <b>`</b> •3	3									TAN AND GRAY, SILTY C (A-2-5) WITH SOME O	LAYEY SAI	
	+	_						/			• •							<u>253.4</u> (WOÓD/TIMBER FRAC		6.5
	251.4	- 8.5						<u> </u>									////	<b>RESIDUAL</b> GRAY AND RED-TAN, SI	TY CLAYE	 -y
250		-	22	11	10	<u>                                     </u>	<b>•</b> /2	1			• •	<u> </u>	· · ·				///	SAND (A-2-6) WITH TRA		
		<del>-</del> -				: :	į	: :									////	247.9		12.0
	246.4	13.5	40		40		ļ											GRAY AND BROWN, F. S CLAY (A-7)	ANDY SILT	ry == <b></b>
245		_	19	8	16		•	24		<u> </u>	• •	ŀ						<u> </u>		
	‡	- -				: :		ļ				ļ_ <u>:</u> _	 					- _ 242.9	2014	17.0
040	241.4	18.5	60/0.1			: :		: :					60/0 1 <sup>4</sup>				\$/LD	- <u>241.3</u> TAN, TRIASSIC ARKOSIC		NE / 18.6
240	-	= =	55,5.1			<u> </u>		<del> </del>				l .	- 5.5.1		RS-1			NON-CRYSTALLINE VERY SLIGHT WEATHER	ROCK	
	1	-										.						<u>- <sup>237.8</sup> </u>	SANDSTO	NE - 22.1
235		-																- WITH WIDE FRACTURE - INDURATED	SPACING	,
200	+	-													RS-2 /			REC = 100%, RQD = 100		
		<b>-</b> -				: :												MODERATE TO SLIGHT V MEDIUM HARD TO MO	DERATELY	
230		-																HARD, RED-BROWN A TRIASSIC SANDSTONE WI		то
		<del>-</del> -										1.						MODERATELY CLOSE	FRACTURE	[
	-	<u>-</u> -				<u> </u>		<u> </u>				<u> </u>		+				_ 227.8 SPACING, MODERATELY - WITH INTERMITTENT A	ND CLAYE	Υ ΄
		- -															<u> </u>	- MUDSTONE ZONES (22. - 27.6 - 28.2), 2 FRACTURE		
		<u>-</u>																DEGREES AND 5 FRACTU	JRES AT 5	
		-																- REC = 100%, RQD = 99	%, GSI = 80	
		- -																Boring Terminated at Eleva NCR: Triassic Arkosic		t IN
		- -																- INCIN. THASSIC AIROSICS	zanasione	
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# GEOTECHNICAL BORING REPORT CORE LOG

SHEET 10

WBS	45632	2.1.1			TIP	B-567	7	С			SRANVIL		GEOLOGIST Goodnigh	nt. D. J.		
			N BRI	DGE NO.								EDGE CREEK		,		ID WTR (ft)
	ING NO						15+00			_ ·	FSET 6		ALIGNMENT -L-		0 HR.	N/A
	LAR EL						<b>PTH</b> 32.	1 ft				856,620	<b>EASTING</b> 2,086,130		-	0.4 Caved
				TE TRI80	l				16			DRILL METHOD Was				Automatic
	LER E						TE 02/0			СС		E 02/02/17	SURFACE WATER DEI			
_	E SIZE	•					<b>N</b> 13.5 f			-						
ELEV	RUN	DEPTH	RUN	DRILL		JN	SAMP.		ATA RQD	L						
(ft)	ELEV (ft)	(ft)	(ft)	RATE (Min/ft)	(ft) %	(ft) (%	NO.	(ft) %	(ft) %	O G	ELEV. (ft)		ESCRIPTION AND REMARK	S		DEPTH (ft)
241.27													Begin Coring @ 18.6 ft			
240	241.3	18.6	3.5	1:33/.5/0.5 2:59/1.0 2:50/1.0 3:09/1.0	(3.5) 100%	(3.5) 100%	RS-1	(3.5) 100%	(3.5) 100%	蠥	241.3	VERY SLIGHT WE	NON-CRYSTALLINE ROCK EATHERING, HARD, TAN, T		C ARKOS	18.6
	237.8	22.1		2.50/1.0 3.09/1.0	(5.0)					囂	237.8	SANDSTONE WI	TH WIDE FRACTURE SPACE = 100%, RQD = 100%, GSI	CING, IN		
235	-	-	5.0	2:30/1.0	(5.0) 100%	(4.9) 98%		(10.0) 100%		菫	<u> </u>	MODERATE TO	SLIGHT WEATHERING, M	EDIUM	HARD TO	
233				2:03/1.0 2:08/1.0			RS-2			蓋	_	WITH CLOSE TO	RED-BROWN AND GRAY, MODERATELY CLOSE FRA	ACTURE	: SPACIN	G,
	232.8	27.1	5.0	2:10/1.0 2:05/1.0		(5.0)				蠹	_	MUDSTONE ZONES	DURATED, WITH INTERMIT (22.7 - 23.7 AND 27.6 - 28.2)	). 2 FRA	CTURES	AT 30
230	_	_		4:16/1.0 2:30/1.0	100%	100%					_	TO 40 DEGREES	S AND 5 FRACTURES AT 5 C = 100%, RQD = 99%, GSI	TO 10 E	DEGREES	S.
	227.8	32.1		2:45/1.0 2:49/1.0						蠥	227.8					32.1
	-										_ E	Boring Terminated at E	levation 227.8 ft IN NCR: Tri	iassic Ar	kosic San	dstone
	-	_									_					
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	-										_					
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NCDOT CORE SINGLE GIBOUR: 01 B-3077 BRIDGE #42 LUGS.GFJ NC_DOT.GDT 9/2/1777	-	-									<u> </u>					
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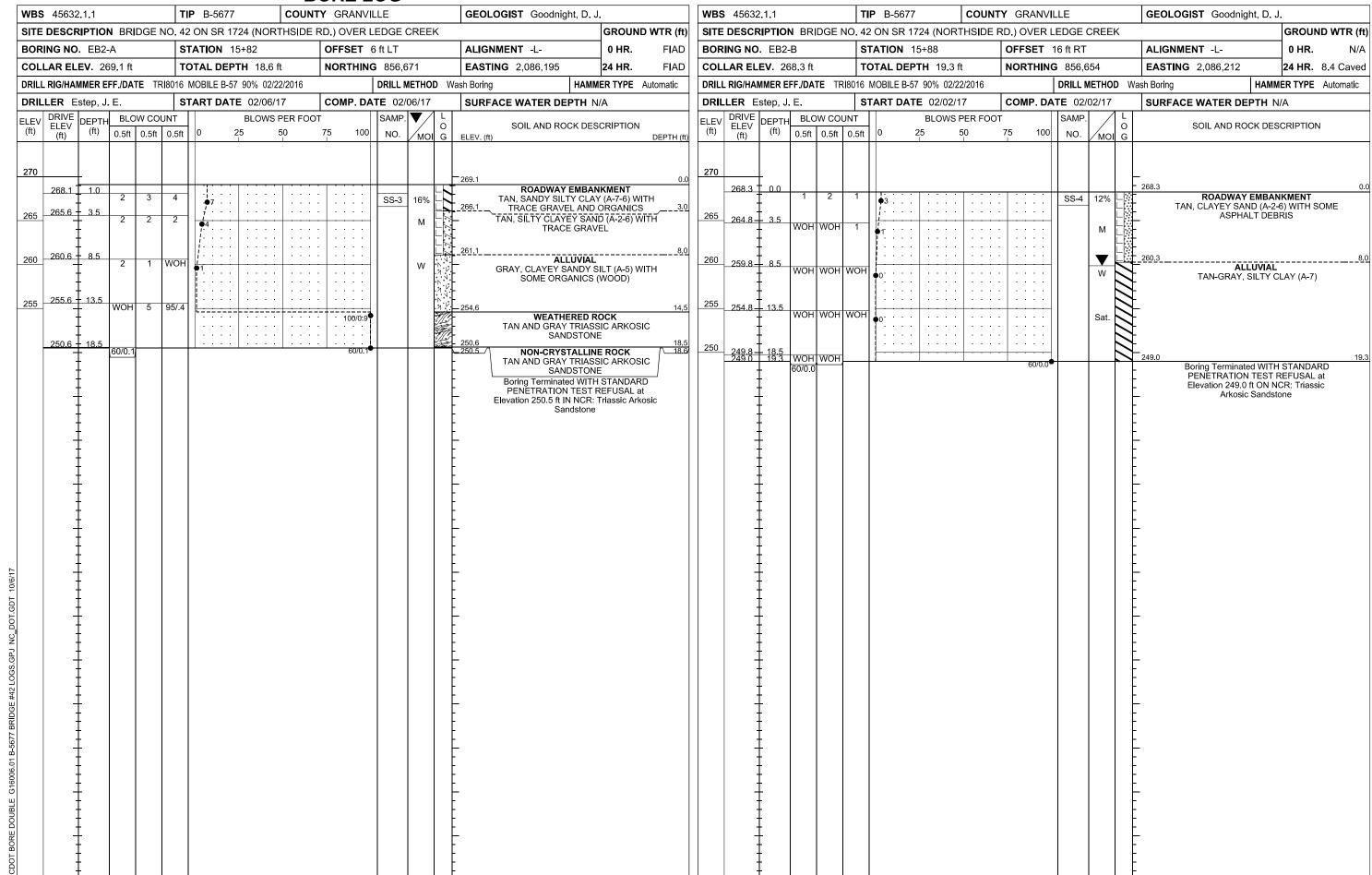
FALCON ENGINEERING, INC. 1210 TRINITY ROAD, SUITE 110 RALEIGH, NC 27607

> PHONE: 919.871.0800 FAX: 919.871.0803

### **ROCK CORE PHOTOGRAPHS**

BRIDGE NO. 42 ON SR 1727 (NORTHSIDE RD.)
OVER LEDGE CREEK
GRANVILLE COUNTY, NC
WBS NO.: 45632.1.1 & TIP NO.:B-5677
FALCON PROJECT NO.: G16006.01

# GEOTECHNICAL BORING REPORT BORE LOG



### UNIAXIAL COMPRESSIVE STRENGTH OF INTACT ROCK CORE SPECIMENS



Performed in General Accordance with ASTM D7012

March 3, 2017

Project Name: B-5677 Br. 42 Project Number: G16006.01

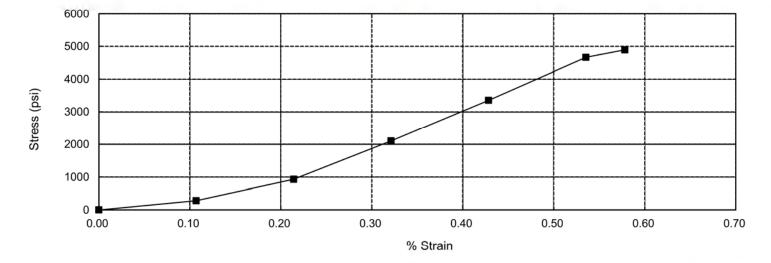
Sample ID.: RS-1 Location: B1-B Depth (ft): 19.3-19.8 Length (in.): 4.67 Diameter (in.): 1.98

Area (in²): 3.079

L/D 2.36 Unit Weight (pcf): 145.6

Compressive Strength (psi): 4890 Time to Failure, mins:sec: 4:40

Deflection (in.)	Strain (%)	Load (lbf)	Strength (psi)	Young's Modulus (psi)
0.000	0.00	0		
0.005	0.11	860	280	261,520
0.010	0.21	2850	930	434,310
0.015	0.32	6470	2100	653,800
0.020	0.43	10310	3350	782,225
0.025	0.54	14350	4660	870,488
0.027	0.58	15070	4890	845,789





\*Young's modulus is calculated using the secant modulus at each data point per Figure 2 (C) in ASMTM D 7012

Reviewed by: John Railly



### UNIAXIAL COMPRESSIVE STRENGTH OF INTACT ROCK CORE SPECIMENS



Performed in General Accordance with ASTM D7012

March 3, 2017

Project Name: B-5677 Br. 42 Project Number: G16006.01

Sample ID.: RS-2 Location: B1-B Depth (ft): 25.1-25.6

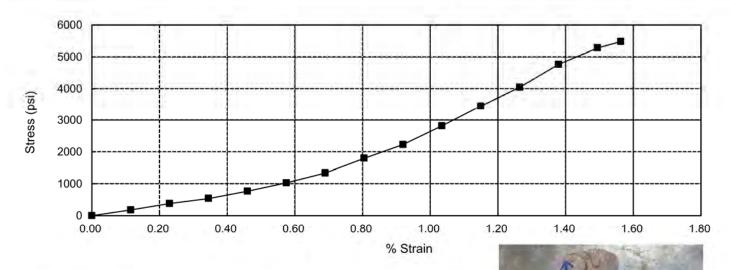
Length (in.): 4.35 Diameter (in.): 1.97 Area (in²): 3.048

L/D 2.21

Unit Weight (pcf): 152.1

Compressive Strength (psi): 5480 Time to Failure, mins:sec: 4:55

Deflection (in.)	Strain (%)	Load (lbf)	Compressive Strength (psi)	Young's Modulus (psi)
0.000	0.00	0		
0.005	0.11	540	180	156,600
0.010	0.23	1150	380	165,300
0.015	0.34	1650	540	156,600
0.020	0.46	2340	770	167,475
0.025	0.57	3150	1030	179,220
0.030	0.69	4090	1340	194,300
0.035	0.80	5480	1800	223,714
0.040	0.92	6790	2230	242,513
0.045	1.03	8590	2820	272,600
0.050	1.15	10530	3450	300,150
0.055	1.26	12350	4050	320,318
0.060	1.38	14540	4770	345,825
0.065	1.49	16090	5280	353,354
0.068	1.56	16690	5480	350,559



\*Young's modulus is calculated using the secant modulus at each data point per Figure 2 (C) in ASMTM D 7012